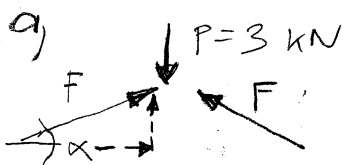


1



$$\alpha = \arctan \frac{9,4}{5,0} = 4,57^\circ$$

$$F = \frac{P}{2 \cdot \sin \alpha} = 18,8 \text{ kN}$$

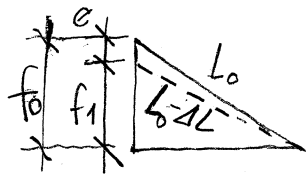
$$\sigma = \frac{F}{A} = \frac{188 \cdot 10^3}{100} = 188 \frac{\text{N}}{\text{mm}^2}$$

$$\varepsilon = \frac{F}{AE} = \frac{188 \cdot 10^3}{100 \cdot 200 \cdot 10^3} = 9,4 \cdot 10^{-4} = 0,94 \text{‰}$$

$$\Delta L = \varepsilon L_0 = 9,4 \cdot 10^{-4} \cdot \sqrt{0,4^2 + 5^2} = 9,4 \cdot 10^{-4} \cdot 5,0160 = 4,715 \cdot 10^{-3} \text{ m}$$

$$f_1 = \sqrt{(L - \Delta L)^2 - L^2} = \sqrt{(5,0160 - 9,0048)^2 - 5^2} = 0,335 \text{ mm}$$

$$f_1 = 0,400 - 0,217 = 0,183 \text{ m}$$



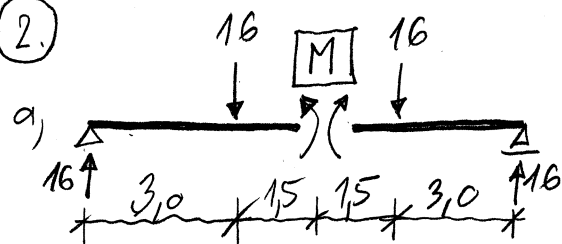
b)

i	f <sub>i</sub> [mm]	L <sub>i</sub> [m]	F <sub>i</sub> [kN]	σ <sub>i</sub> [ $\frac{\text{N}}{\text{mm}^2}$ ]	ε <sub>i</sub> [‰]	ΔL [mm]	e <sub>i</sub> [mm]
0	400	5,0160	18,8	188	0,94	4,715	65
1	335	5,0113	22,4	224	1,12	5,604	77
2	329	5,0104	23,3	233	1,17	5,831	81
3	319	5,0102	23,6	236	1,18	5,902	82 = e

→ 318 mm = f

c) több mint duplája ΔL = 9,43 mm, L<sub>1</sub> = 5,006 m → 5 m átvett

2



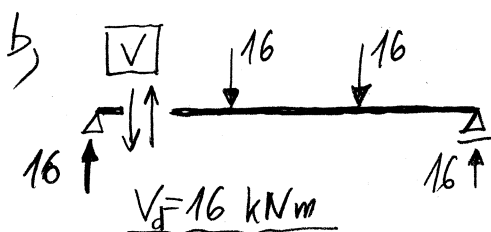
$$M_d = 16(4,5 - 1,5) = 48 \text{ kNm}$$

$$H = \frac{M}{z} = \frac{48}{0,23} = 208,7 \text{ kN}$$

$$\sigma_d = \frac{H}{A_{\text{öv}}} = \frac{208,7 \cdot 10^3 \text{ N}}{10 \cdot 120 \text{ mm}^2} = 174 \frac{\text{N}}{\text{mm}^2} \leq f_y = 235 \text{ MPa} \quad \text{MF}$$

Vagy:  $M_{R,d} = f_y \cdot b \cdot t_{\text{öv}} (h - t_{\text{öv}}) = 235 \cdot 120 \cdot 10 (240 - 10) = 64,86 \cdot 10^6 \text{ Nmm}$

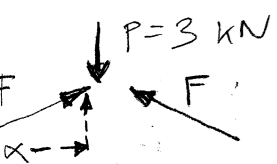
$M_{R,d} = 64,86 \text{ kNm} \geq M_d = 48 \text{ kNm} \quad \text{MF}$



$$\tau_d = \frac{V_d}{A_{\text{öv}}} = \frac{16 \cdot 10^3 \text{ N}}{6 \cdot 220 \text{ mm}^2} = 12,12 \text{ N/mm}^2$$

$$\tau_d = 12,12 \frac{\text{N}}{\text{mm}^2} \ll \tau_y = 135 \frac{\text{N}}{\text{mm}^2} \quad \text{MF}$$

c) felső öv kiferdülés, felső öv horpadás, gerinchorpadás



$$\alpha = \arctan \frac{94}{50} = 4,57^\circ$$

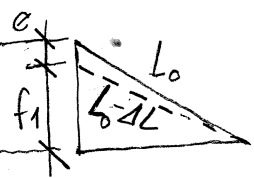
$$F = \frac{P}{2 \cdot \sin \alpha} = 18,8 \text{ kN} \quad \sigma = \frac{F}{A} = \frac{188 \cdot 10^3}{100} = 188 \frac{\text{N}}{\text{mm}^2}$$

$$\varepsilon = \frac{F}{AE} = \frac{188 \cdot 10^3}{100 \cdot 200 \cdot 10^3} = 9,4 \cdot 10^{-4} = 0,94 \text{‰}$$

$$\Delta L = \varepsilon L_0 = 9,4 \cdot 10^{-4} \cdot \sqrt{94^2 + 5^2} = 9,4 \cdot 10^{-4} \cdot 5,0160 = 4,715 \cdot 10^{-3} \text{ m}$$

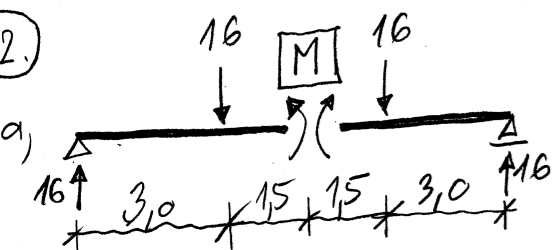
$$f_1 = \sqrt{(L - \Delta L)^2 - L^2} = \sqrt{(5,0160 - 9,43 \cdot 10^{-3})^2 - 5^2} = 9,335 \text{ mm}$$

$$f_1 = 9,400 - 9,217 = 0,183 \text{ m}$$



i	f <sub>i</sub> [mm]	L <sub>i</sub> [m]	F <sub>i</sub> [kN]	σ <sub>i</sub> [N/mm <sup>2</sup> ]	ε <sub>i</sub> [‰]	ΔL [mm]	e <sub>i</sub> [mm]
0	400	5,0160	18,8	188	0,94	4,715	65
1	335	5,0113	22,4	224	1,12	5,604	77
2	329	5,0104	23,3	233	1,17	5,831	81
3	319	5,0102	23,6	236	1,18	5,902	82 = e

→ 318 mm = f  
több mint duplája ΔL = 9,43 mm, L<sub>1</sub> = 5,006 m → 5 m ártatlan



$$M_d = 16(4,5 - 1,5) = 48 \text{ kNm}$$

$$H = \frac{M}{z} = \frac{48}{0,23} = 208,7 \text{ kN}$$

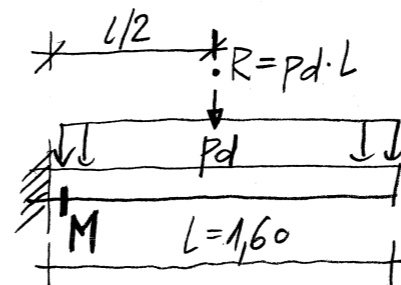
$$\sigma_d = \frac{H}{A_{öv}} = \frac{208,7 \cdot 10^3 \text{ N}}{10 \cdot 120 \text{ mm}^2} = 174 \frac{\text{N}}{\text{mm}^2} \leq f_y = 235 \quad \text{MF}$$

Vagy:  $M_{R,d} = f_y \cdot b \cdot t_{öv} (h - t_{öv}) = 235 \cdot 120 \cdot 10 (240 - 10) = 64,86 \cdot 10^6 \text{ Nmm}$   
 $M_{R,d} = 64,86 \text{ kNm} \geq M_d = 48 \text{ kNm} \quad \text{MF}$

b)  $\tau_d = \frac{V_d}{A_{öv}} = \frac{16 \cdot 10^3 \text{ N}}{6 \cdot 220 \text{ mm}^2} = 12,12 \text{ N/mm}^2$   
 $\tau_d = 12,12 \frac{\text{N}}{\text{mm}^2} \ll \tau_y = 135 \frac{\text{N}}{\text{mm}^2} \quad \text{MF}$

c) felső öv kiferdulása, felső öv horpadása, gerinchorpadás

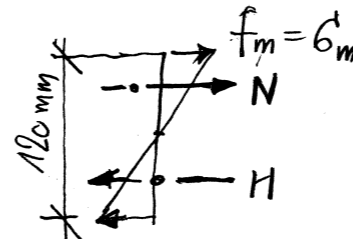
3.



Terhekből (hatás): "EFFECT"

$$M_{E,d} = Pd \cdot L \cdot \frac{L}{2} = \frac{Pd \cdot L^2}{2}$$

Telherbirds (anyag ellenállása): "RESISTANCE"



1 m széles sávot tekintve

$$M_{R,d} = \frac{bh^2}{6} \cdot f_m = \frac{1000 \cdot 120^2 \text{ mm}^3}{6} \cdot 10 \frac{\text{N}}{\text{mm}^2}$$

szilárdsági feltétel:  $M_{R,d} = 24 \cdot 10^6 \text{ Nmm} = 24 \text{ kNm} \geq M_{E,d}$

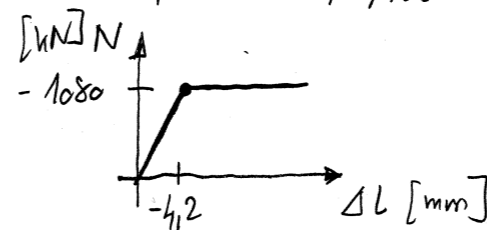
$$\frac{Pd \cdot L^2}{2} \leq M_{R,d} \quad Pd \leq \frac{2M_{R,d}}{L^2} = \frac{2 \cdot 24}{1,6^2} = 18,75 \frac{\text{kN}}{\text{m}}$$

A felületre:  $Pd \leq 18,75 \frac{\text{kN}}{\text{m}^2}$

4.

a)  $N_1 = f_c \cdot A_c = 12 \frac{\text{N}}{\text{mm}^2} \cdot 300 \text{ mm}^2 = 1,08 \cdot 10^6 \text{ N} = 1080 \text{ kN}$

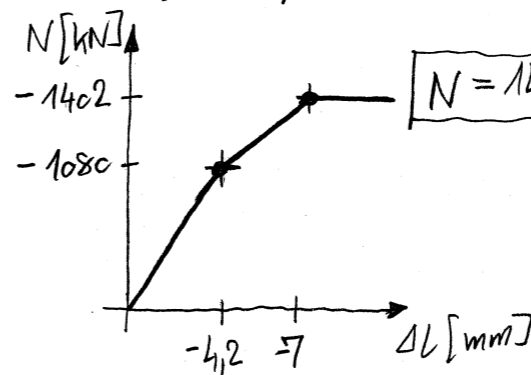
$$\Delta L_1 = \varepsilon \cdot L = 9,12/100 \cdot 3500 \text{ mm} = 4,2 \text{ mm}$$



b)  $N_2 = f_c A_c + f_s A_s = 12 \cdot 300^2 + 400 \cdot 4 \cdot 201 \text{ mm}^2 = 1,402 \cdot 10^6 \text{ N} = 1402 \text{ kN}$

Ez a telherbirds ε = 0,2‰ összenyomódásnál következik be.

$$\Delta L_2 = 0,2/100 \cdot 3500 = 7,0 \text{ mm}$$

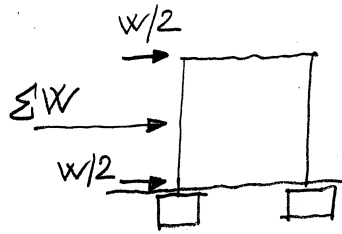
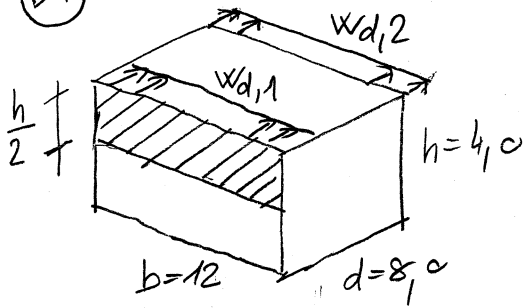


c) keresztmetszet telherbirds

d) a kihajlásveszély miatt nem érhető el a keresztmetszet nyomási telherbirds

→ Oszlop telherbirds kisebb a keresztmetszet telherbirdsánál

5.



a) Torló: III. kat. /  $h=4.0\text{m}$  /  $q_p(z) = 0.446 \text{ kN/m}^2$

Arány:  $h/d = 4/8 = 0.5 \leq 1$   $c_1 = 0.8$   $c_{II} = -0.5$   
 $> 0.25$   $c_1 = 0.7$   $c_{II} = -0.3$

interpoláció: szélnyomás:  $c_1 = 0.7 + 0.1 \cdot \frac{0.25}{0.75} = 0.733$

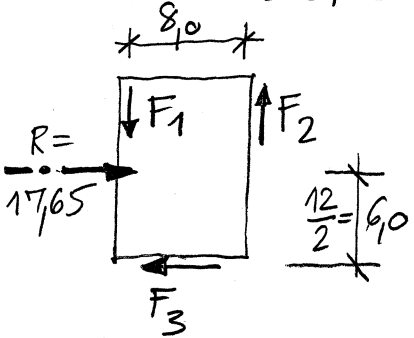
szélszívás:  $c_{II} = -0.3 - 0.2 \cdot \frac{0.25}{0.75} = -0.366$

Széltéher:  $w_{d,1} = \delta q \cdot c_1 \cdot q_p(z) = 1.5 \cdot 0.733 \cdot 0.446 = 0.4904 \text{ kN/m}^2$

$w_{d,2} = \delta q \cdot c_{II} \cdot q_p(z) = 1.5 \cdot 0.366 \cdot 0.446 = 0.2449 \text{ kN/m}^2$

$w_d = 0.7353 \text{ kN/m}^2$

$R_{w,d} = w_d \cdot b \cdot \frac{h}{2} = 0.7353 \cdot 12 \cdot \frac{4}{2} = 17.65 \text{ kN}$



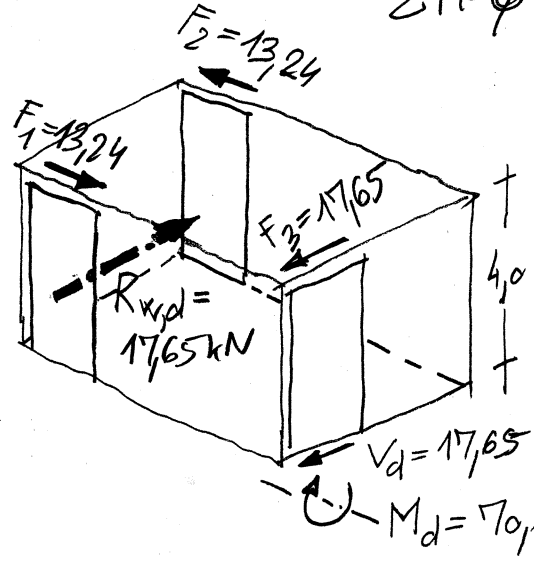
b) Falakra jutó erők - alprajzban:

$\sum X = 0 \rightarrow F_3 = R = 17.65 \text{ kN} (\leftarrow)$

$\sum Y = 0 \rightarrow F_1 = F_2$

$\sum M = 0 \rightarrow R \cdot \frac{b}{2} = F_1 \cdot d$

$F_1 = F_2 = \frac{17.65 \cdot 6.0}{8} = 13.24 \text{ kN} (\downarrow) (\uparrow)$



c) (3) fal reakciói:

$V_d = F_3 = 17.65 \text{ kN} (\leftarrow)$

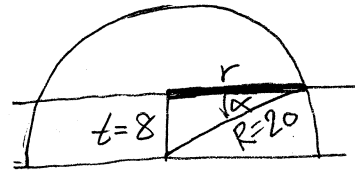
$M_d = F_3 h = 17.65 \cdot 4.0 = 70.6 \text{ kNm} (\curvearrowright)$

6.

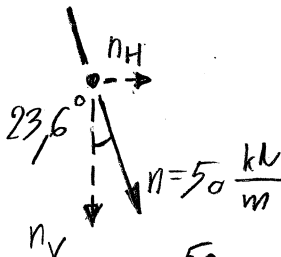
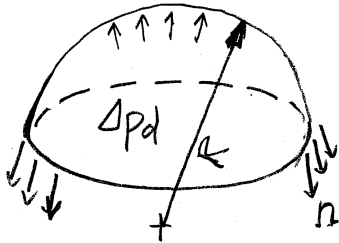
Geometria:

$$r = \sqrt{R^2 - t^2} = \sqrt{20^2 - 8^2} = 18,33 \text{ m}$$

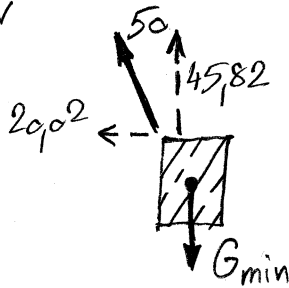
$$\alpha = \arcsin \frac{8}{20} = 23,6^\circ$$



a)



"Fűjt" ponyva támaszerő



Ellenerő: vasbeton alapgyűrűt terheli

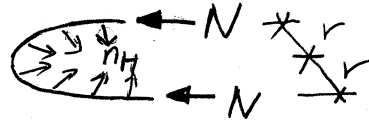
Gömbszeklet-támaszerő érintőirányú

$$n_d = \frac{\Delta p_d \cdot R}{2t} = \frac{\Delta p_d \cdot R}{2} = \frac{5 \text{ kPa} \cdot 20}{2} = 50 \frac{\text{kN}}{\text{m}}$$

$$n_{Hd} = n \cdot \sin 23,6^\circ = 50 \cdot \sin 23,6^\circ = 20,02 \frac{\text{kN}}{\text{m}}$$

$$n_{Vd} = n \cdot \cos 23,6^\circ = 50 \cdot \cos 23,6^\circ = 45,82 \frac{\text{kN}}{\text{m}}$$

b, Vízsz. komp. - gyűrűerő veszi fel



$$N = n_H \cdot r = 20,02 \cdot 18,33 = 367,0 \text{ kN}$$

nyomás!

c, Függőleges komponens: gyűrű önsúlya egyensúlyozza - stabilitás!

$$n_{Vd} \leq \delta_{\min} \cdot G = \delta_{\min} \times \delta_{vb} \times A$$

$$A \geq \frac{n_{Vd}}{\delta_{\min} \times \delta_{vb}} = \frac{45,82 \frac{\text{kN}}{\text{m}}}{0,9 \cdot 25 \frac{\text{kN}}{\text{m}^3}} = 2,04 \frac{\text{m}^3}{\text{m}}$$

$$b = 1,40 / h = 1,50 \text{ m gyűrű}$$

$$b = \frac{A}{h} = \frac{2,04}{1,50} = 1,36 \text{ m}$$